

Chile Earthquake of February 27, 2010

Damage Observations



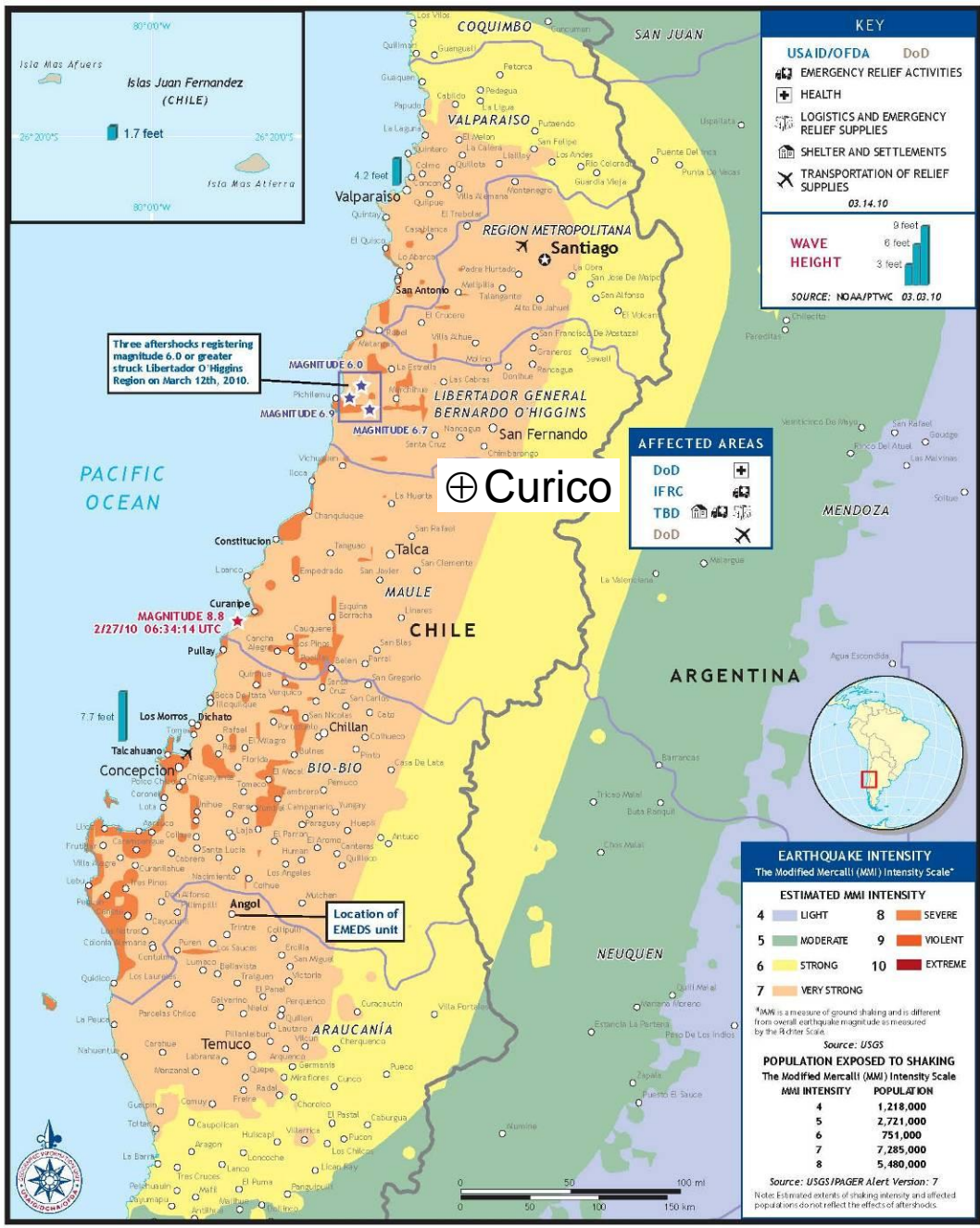
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Majid Sarraf, PARSONS Engineering

Iranian American Society of Engineers and Architects (IASEA)

Irvine, April 7th, 2010

EARTHQUAKE AND TSUNAMI-AFFECTED AREAS IN CHILE USG HUMANITARIAN ASSISTANCE



2010 Chile earthquake

3:34 am local time
27 February 2010

M_w 8.8

Fault rupture 100km x 500 km

Population affected > 8M*

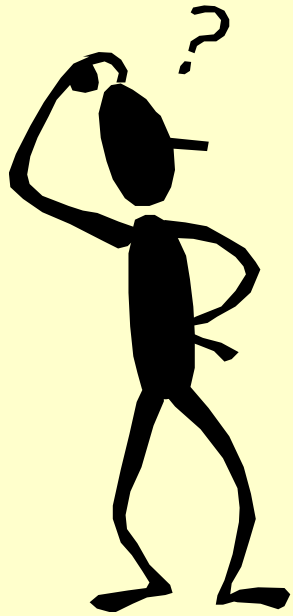
452 confirmed deaths**

~800,000 homeless

~\$30B damages

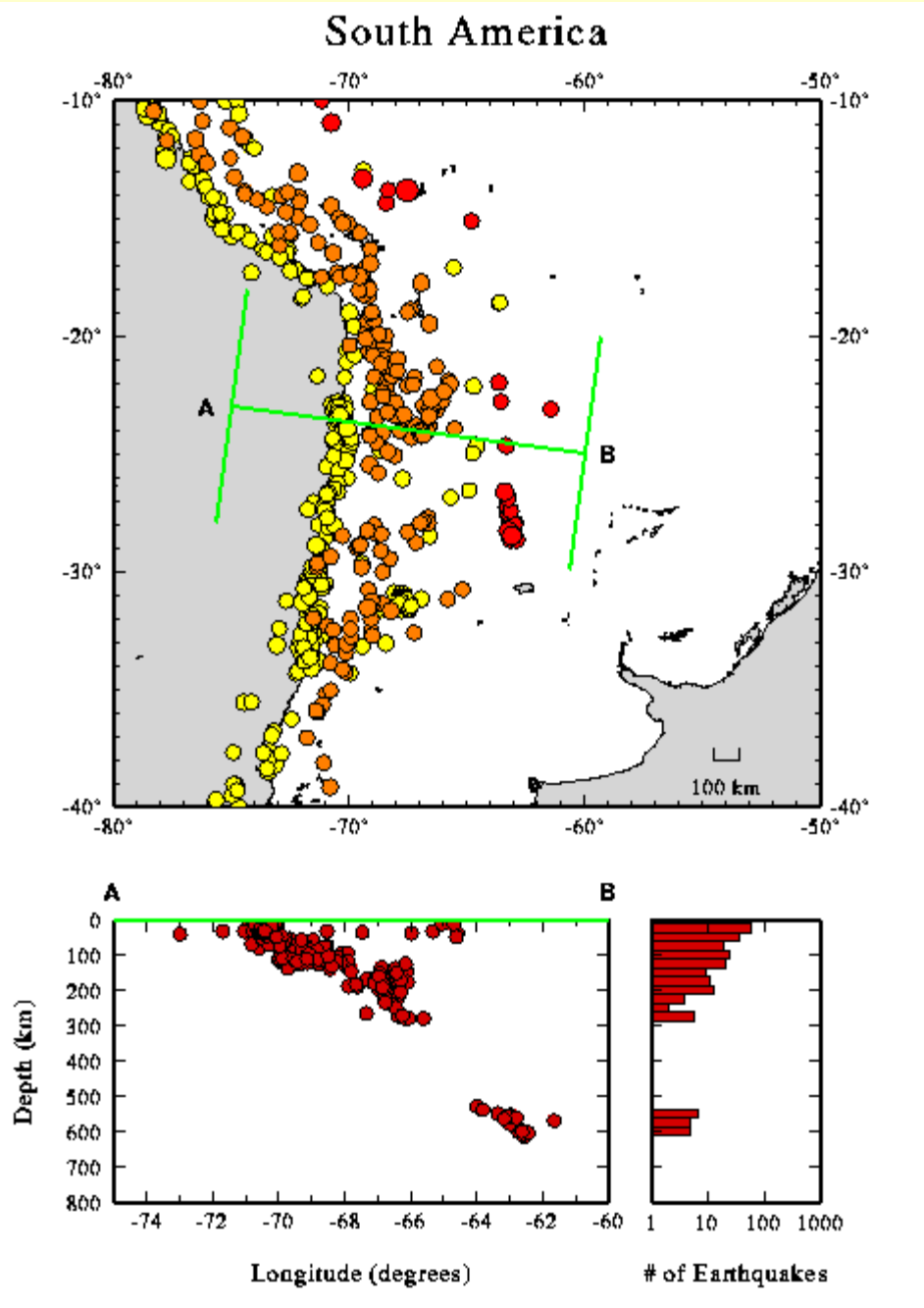
*ACHISINA
**USAID, 3/25/10

Challenges in collapse prediction

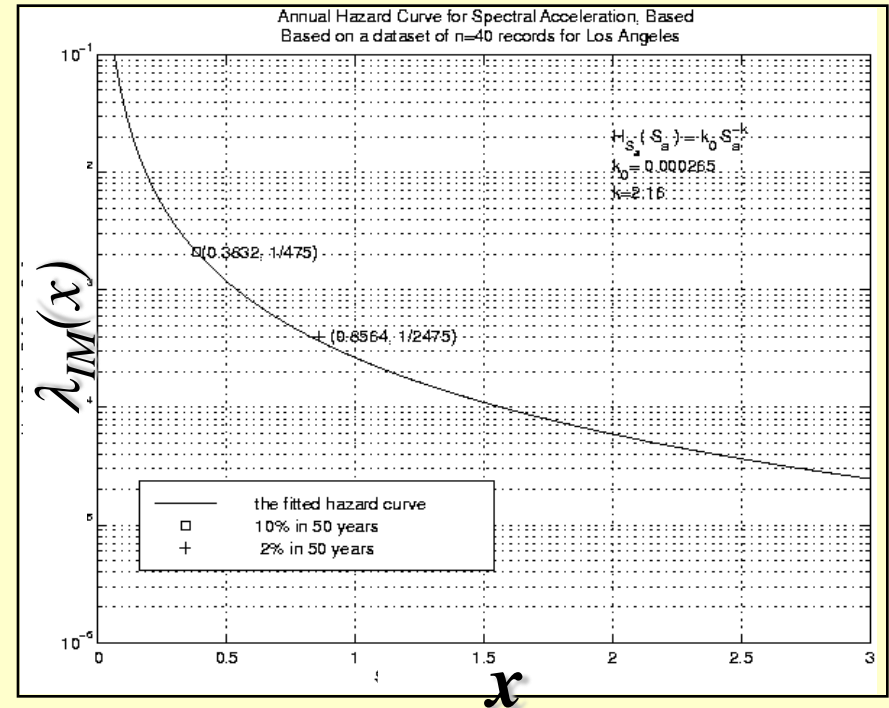
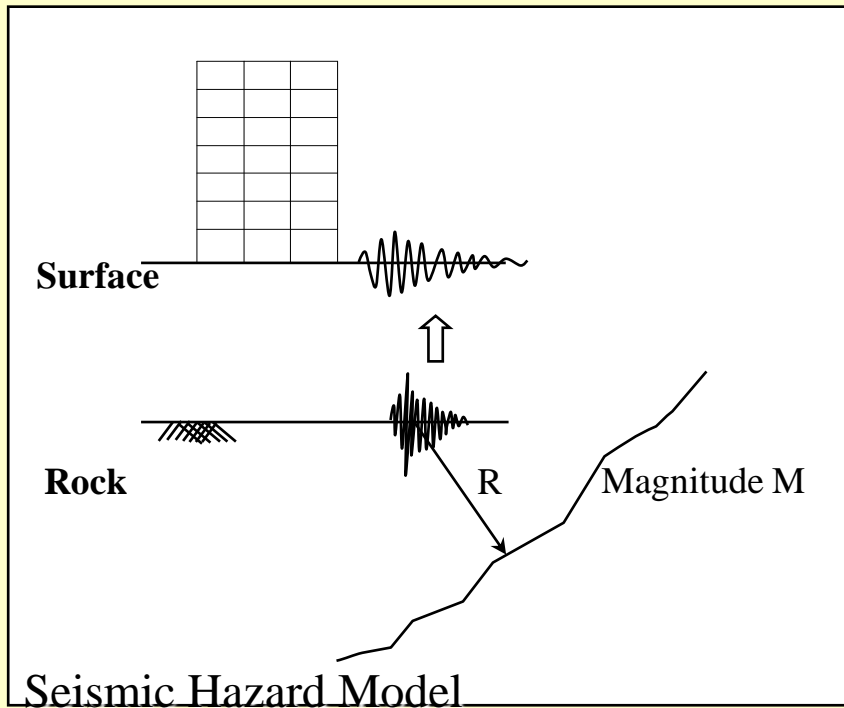


- We'll never be able to do it right
- Even if we knew how, what difference does it make in the context of structural design decisions?
- We can not codify design for collapse safety
- All structures have a finite probability of collapse (because of the uncertainty in the earthquake hazard)
- Most older structures have a significant probability of collapse
- Collapse has economic and social consequences and it should be considered in a performance-based design decision process

Depth of epicenter of earthquakes (1991-1996, CNSS Earthquake Maps).



Hazard analysis → Hazard curve



$\lambda_{IM}(x)$: Mean Annual Rate of Events With Site
Intensity $IM \geq x$

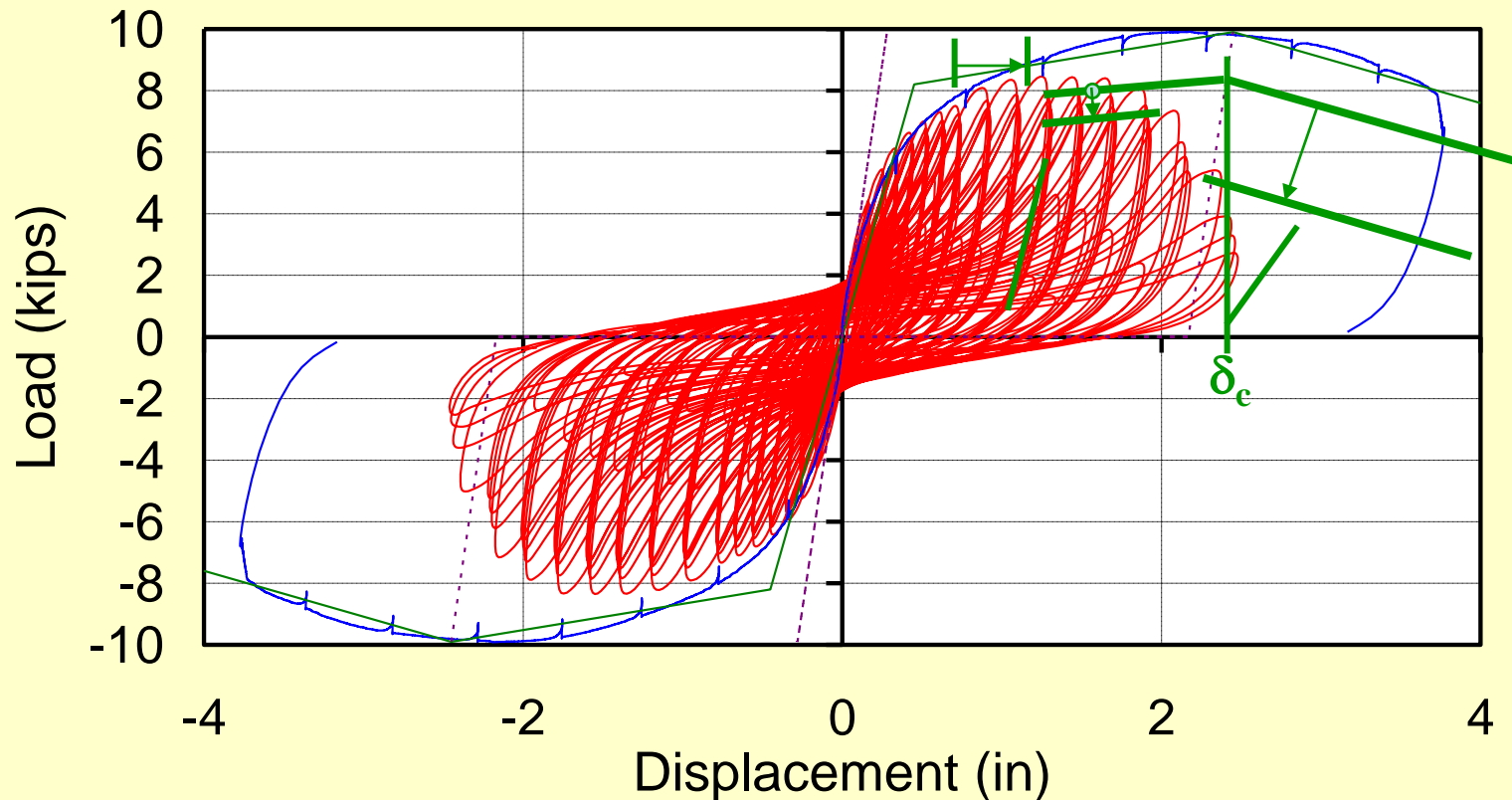
Collapse capacity analysis

1. Develop realistic analytical model of structure that permits prediction of the response till collapse occurs (required modeling of deterioration)
2. Subject model of structure to ground motion of increasing intensity until collapse occurs (evaluate collapse capacity)
3. Subject structure to sufficient number of ground motions to perform statistical evaluation of collapse capacity (obtain collapse fragility curve accounting for aleatory variability)
4. Modify fragility curve for modeling (epistemic variability) uncertainties

Component modeling : Basic modes of deterioration

UCI G12 OSB

$F_y=8.2$ kips, $\delta_y=0.45$ in, $\alpha_s=0.047$, $\alpha_c=-0.081$, $\alpha_u=1.94$, $\delta_c/\delta_y=5.44$

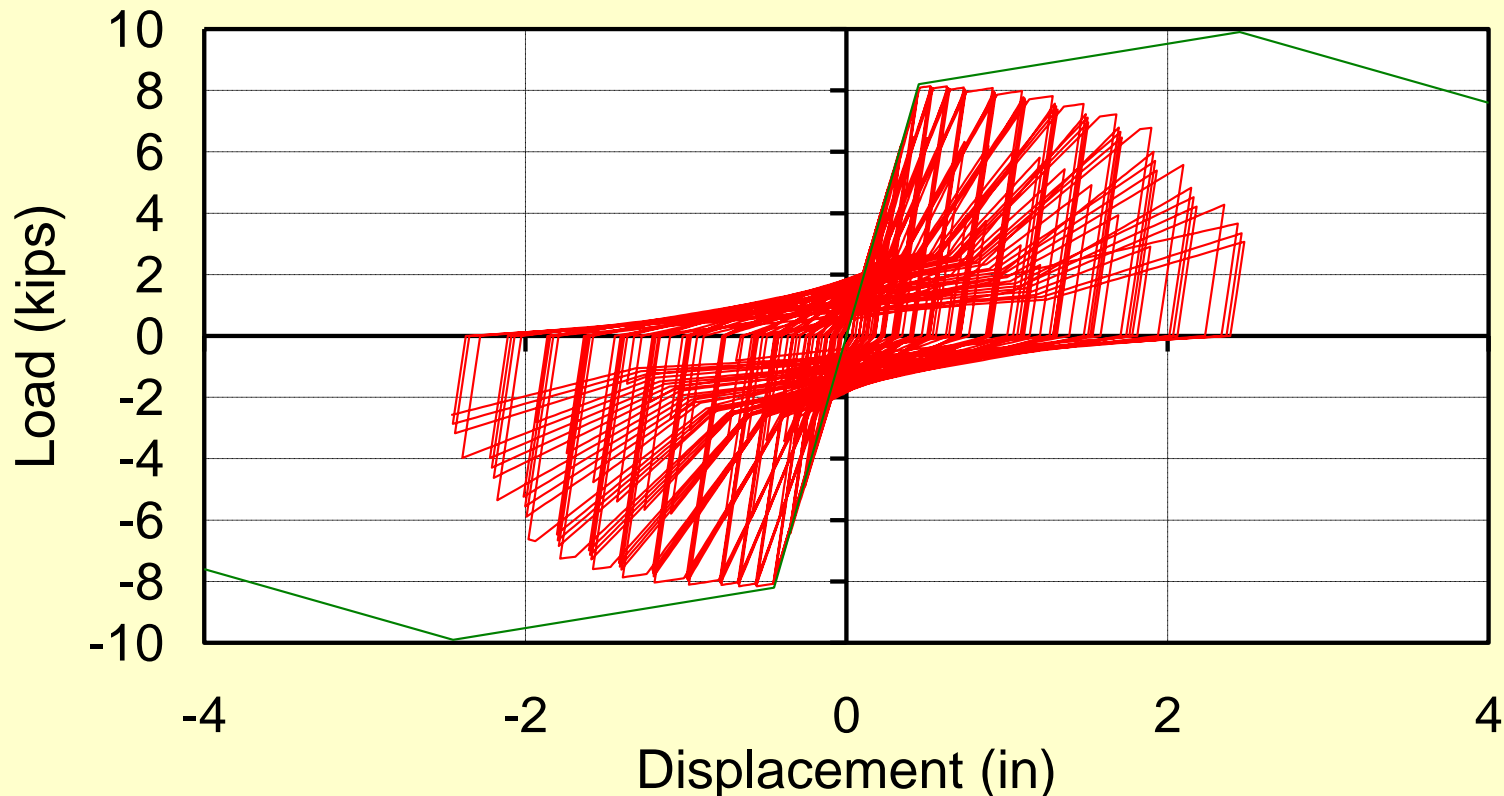


Component modeling : Basic modes of deterioration

UCI G12 OSB

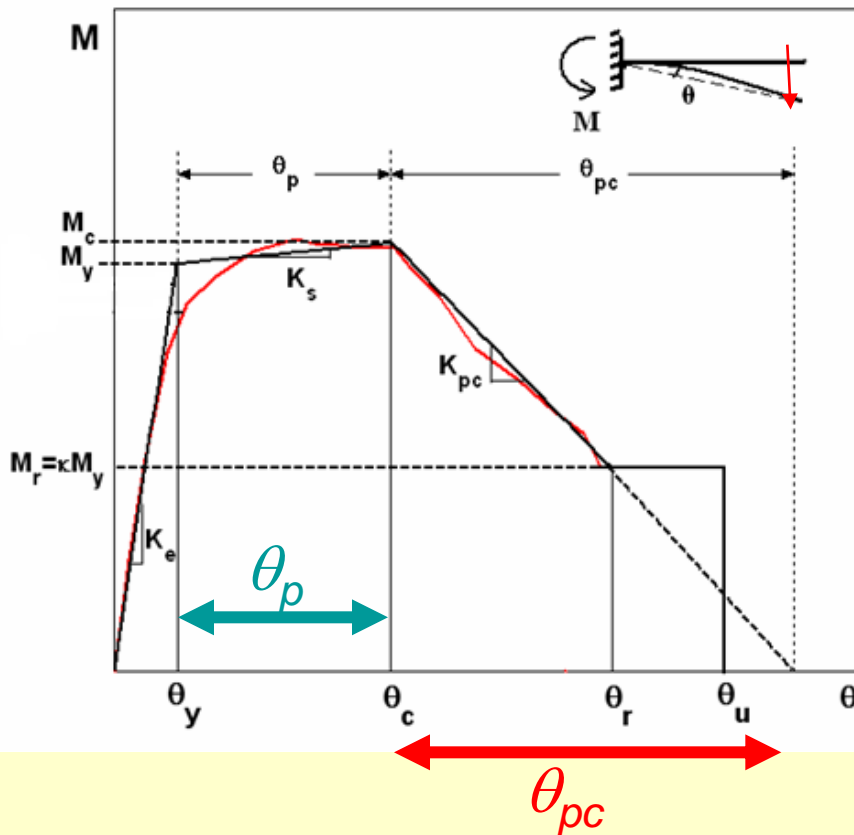
Pinching Model, $\kappa=0.5$, $F_y=8.2$ kips, $\delta_y=0.45$ in

$\alpha_s=0.047$, $\alpha_c=-0.081$, $\alpha_c=1.94$, $\delta_c/\delta_y=5.44$, $\gamma_s=270$, $\gamma_c=270$, $\gamma_k=\infty$, $\gamma_a=270$

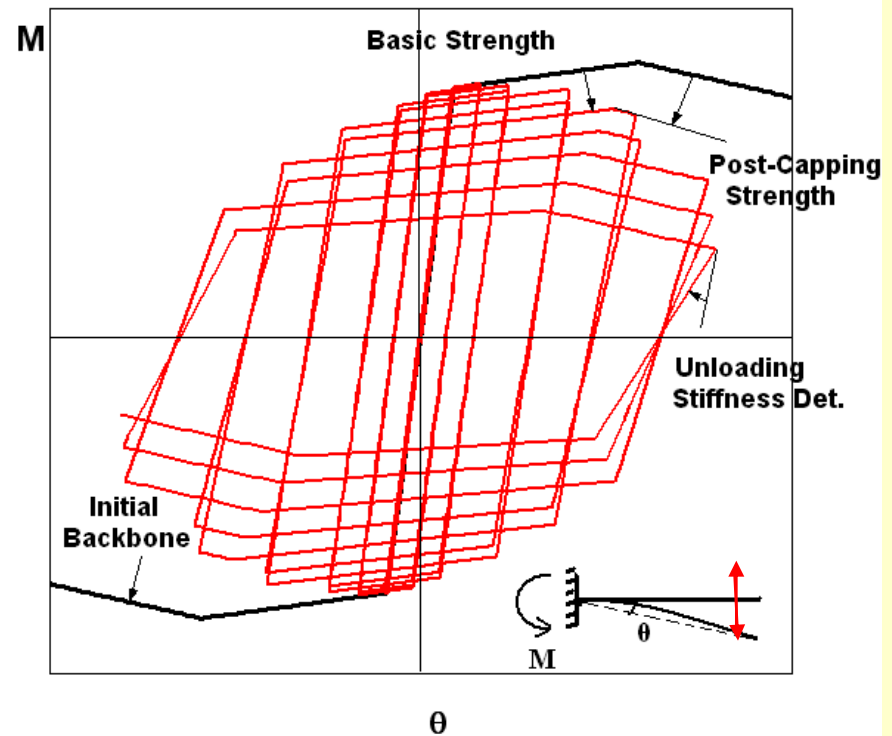


Component modeling : Basic modes of deterioration

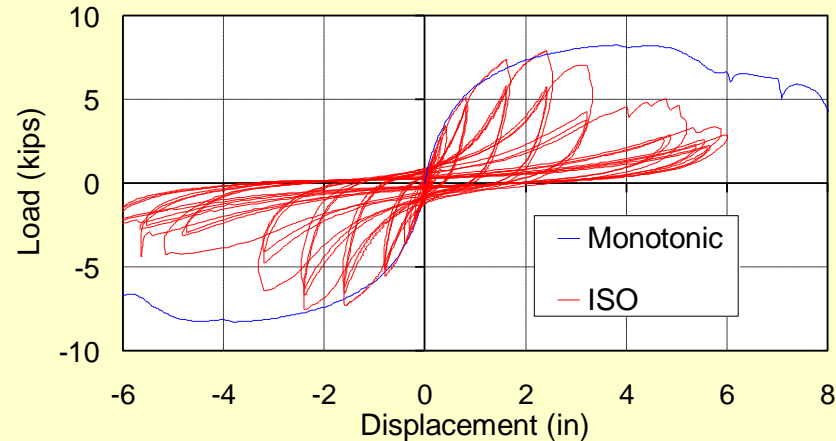
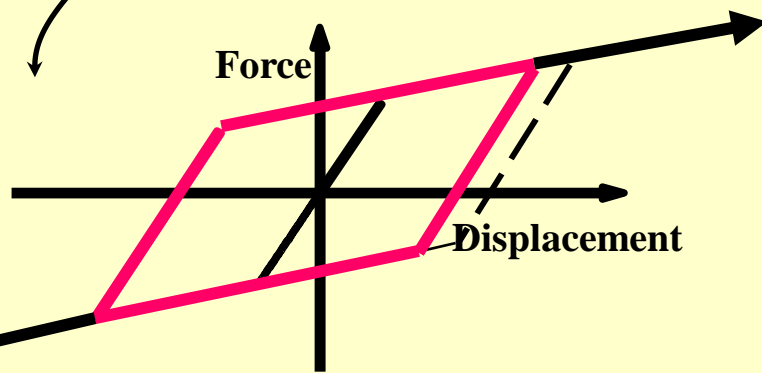
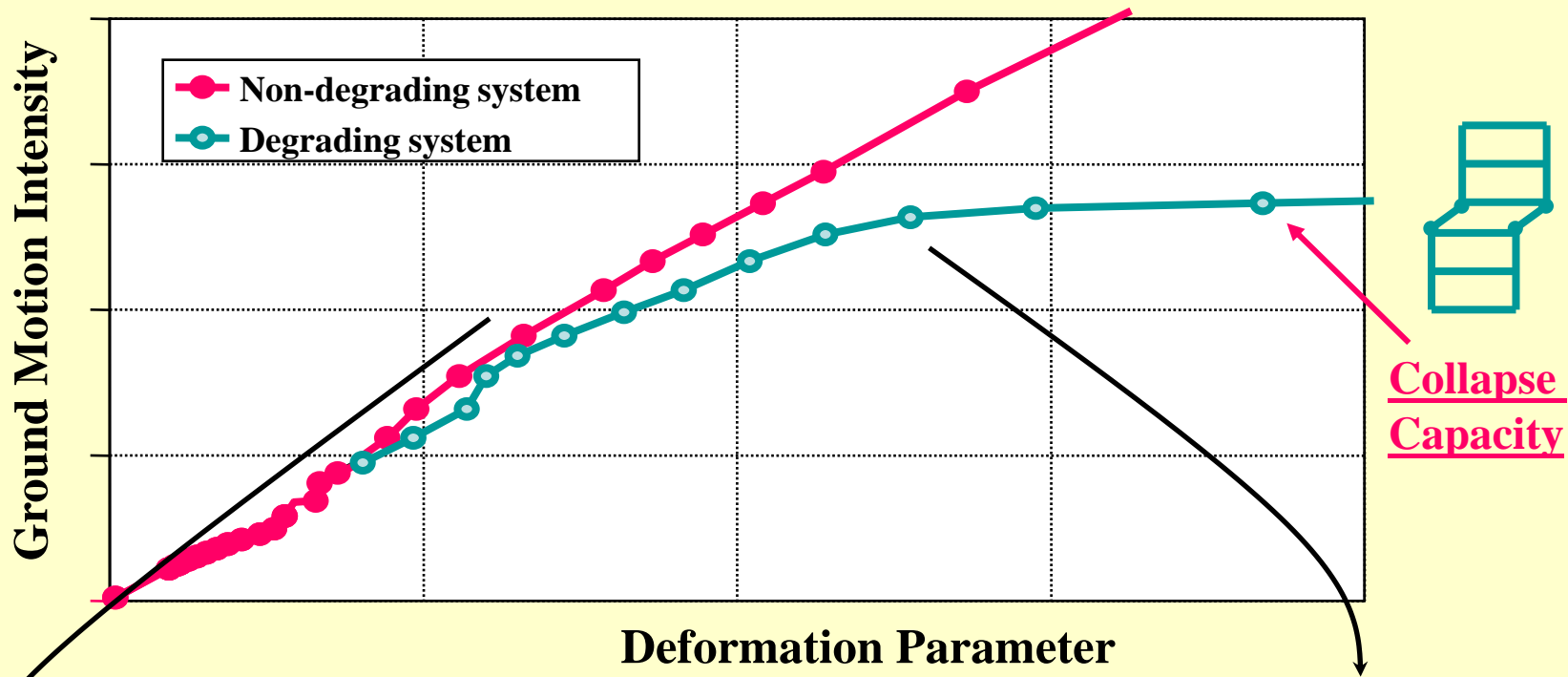
Monotonic Moment-Rotation Model



Cyclic Moment-Rotation Model



Deterioration and Collapse (MDOF)



After H. Krawinkler

Developing Collapse Fragility Curve

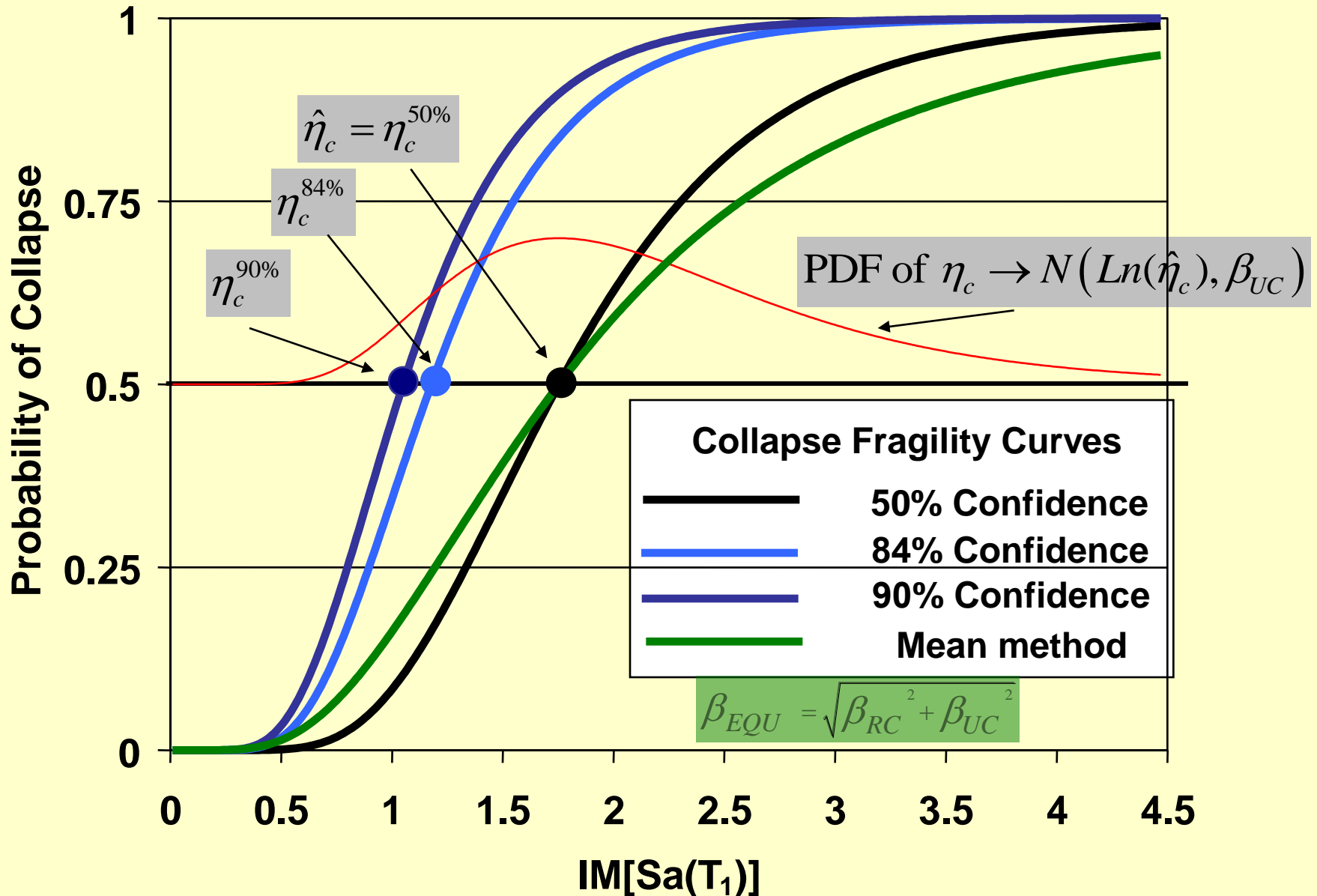
Obtaining the collapse fragility curve (SW)

$N = 8$, $T = 0.8$, $\gamma = 0.25$, Stiff = Unif. Str. = $-0.05M_{y,base}$, $\xi = 0.05$

$\theta_p = 0.02$, $\theta_{pc}/\theta_p = 1$, $\lambda = 20$, $M_c/M_y = 1.1$



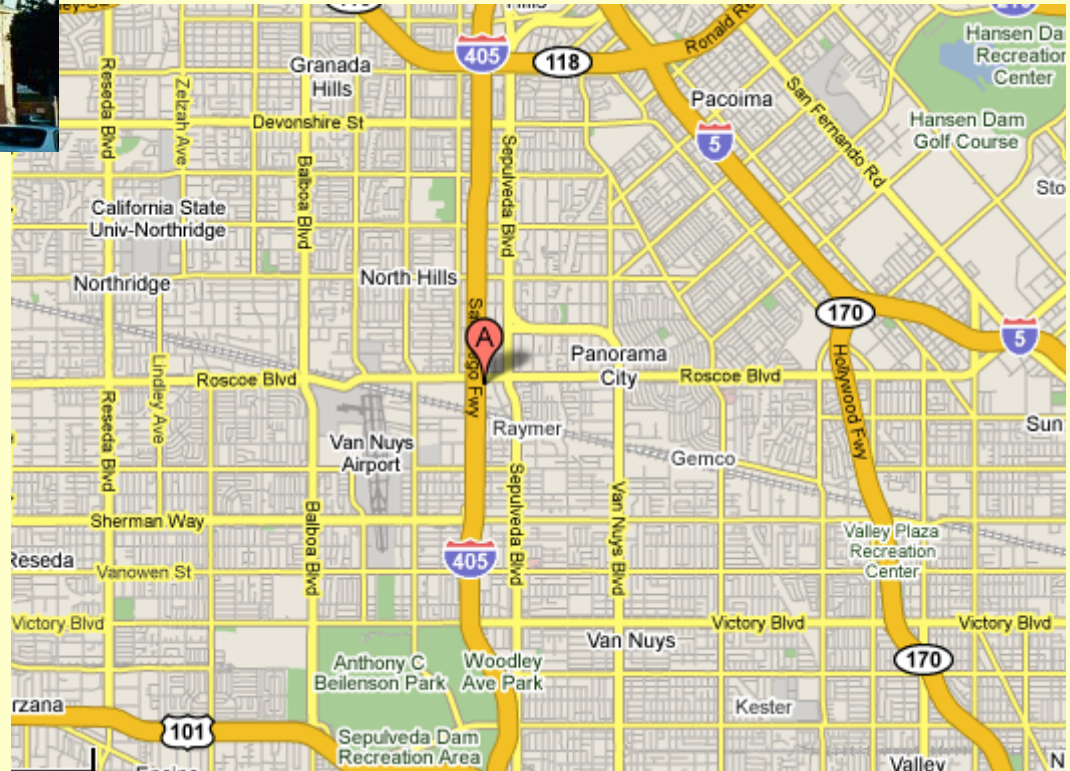
Effect of uncertainty on prob. of collapse



Assessment of probability of collapse at certain hazard level



Van Nuys Hotel Structure

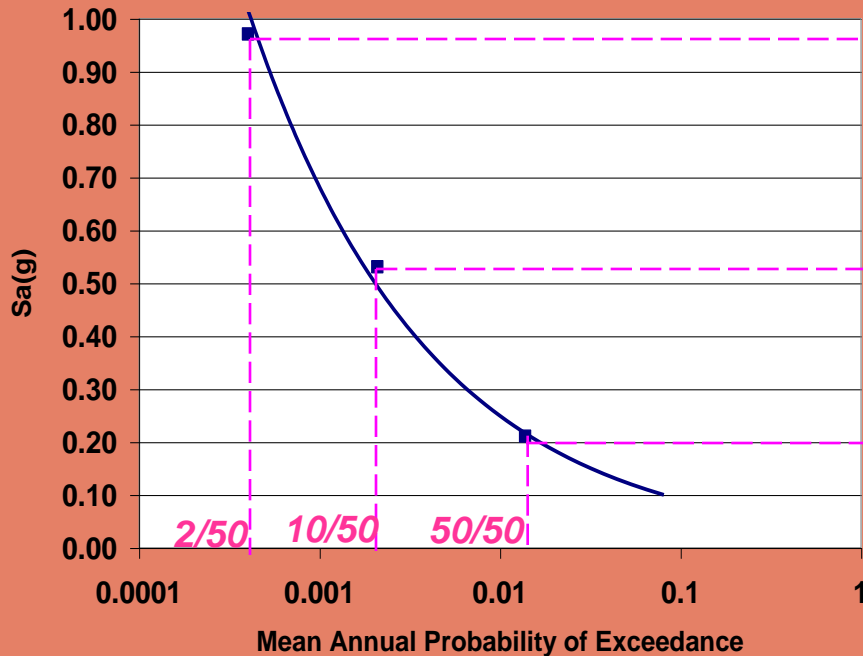


Assessment of probability of collapse at certain hazard level

Hazard Curve

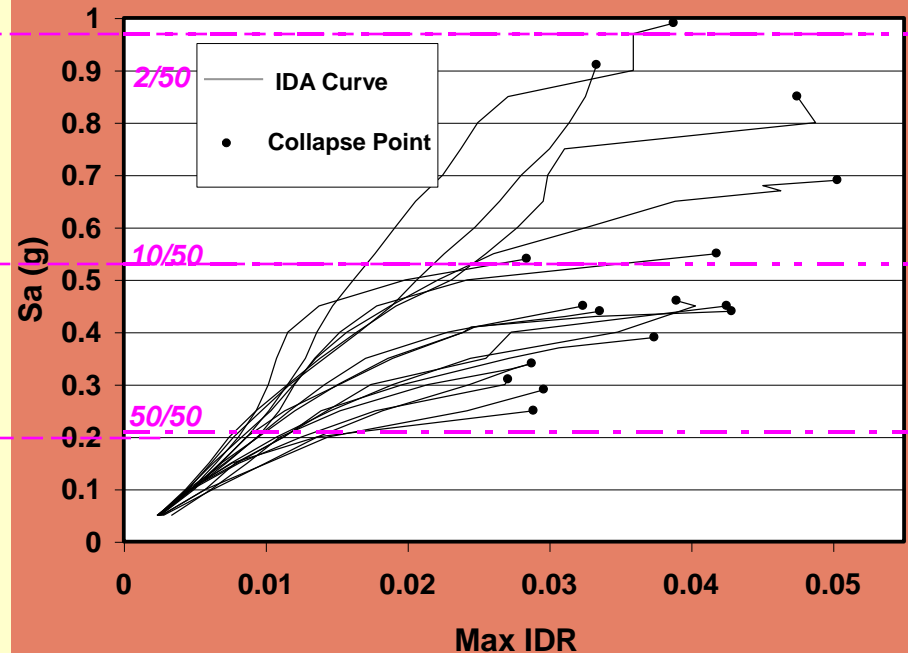
$$\lambda(Sa)$$

Mean Hazard Curve for VNHB, T = 1.5 sec.



Incremental Dynamic Analysis

IDA curves for VNHB Structure

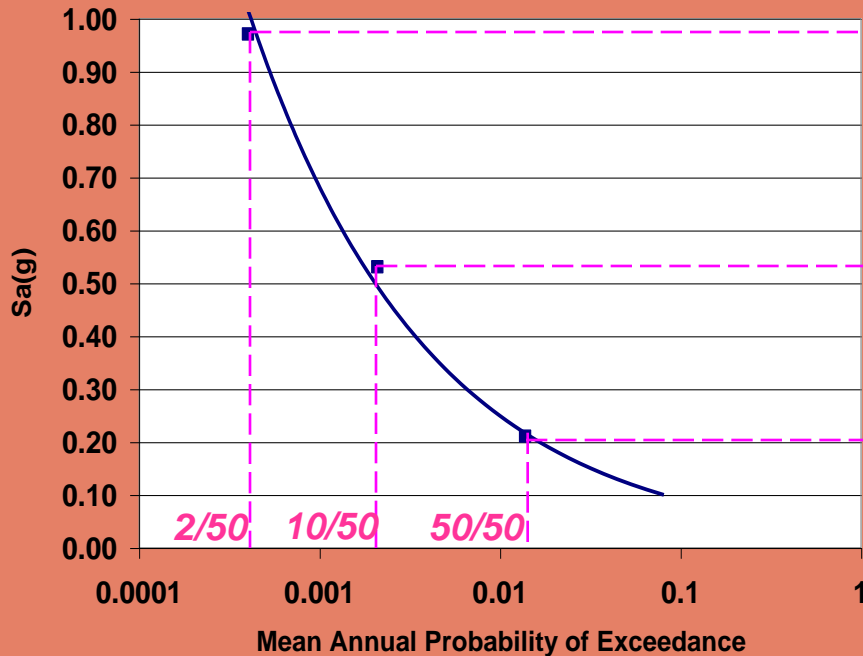


Assessment of probability of collapse at certain hazard level

Hazard Curve

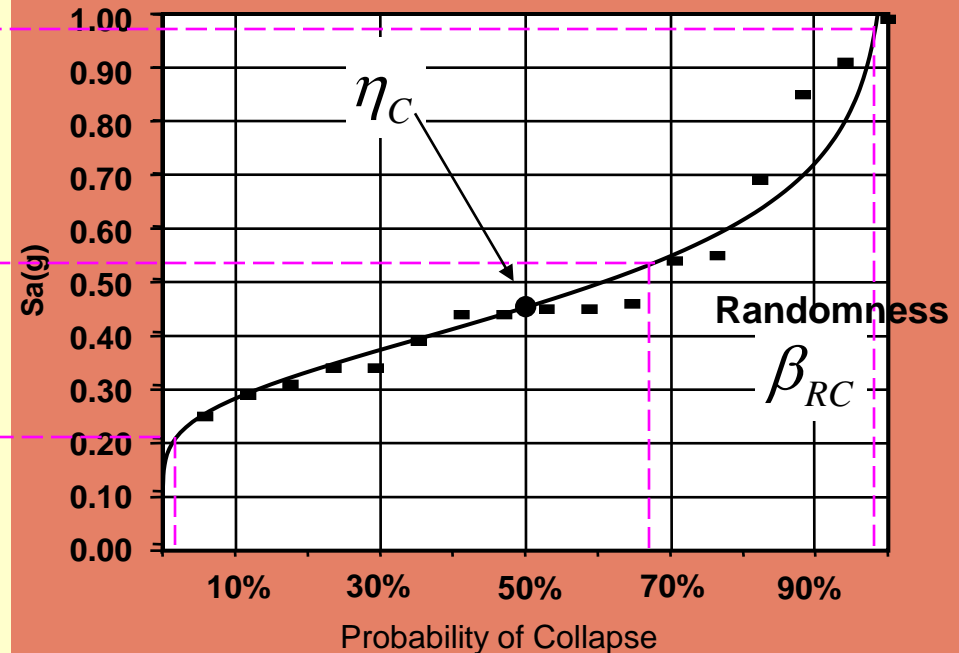
$$\lambda(Sa)$$

Mean Hazard Curve for VNHB, T = 1.5 sec.



Collapse fragility curve

$$P(C | Sa)$$



Assessment of probability of collapse at certain hazard level

Hazard Curve

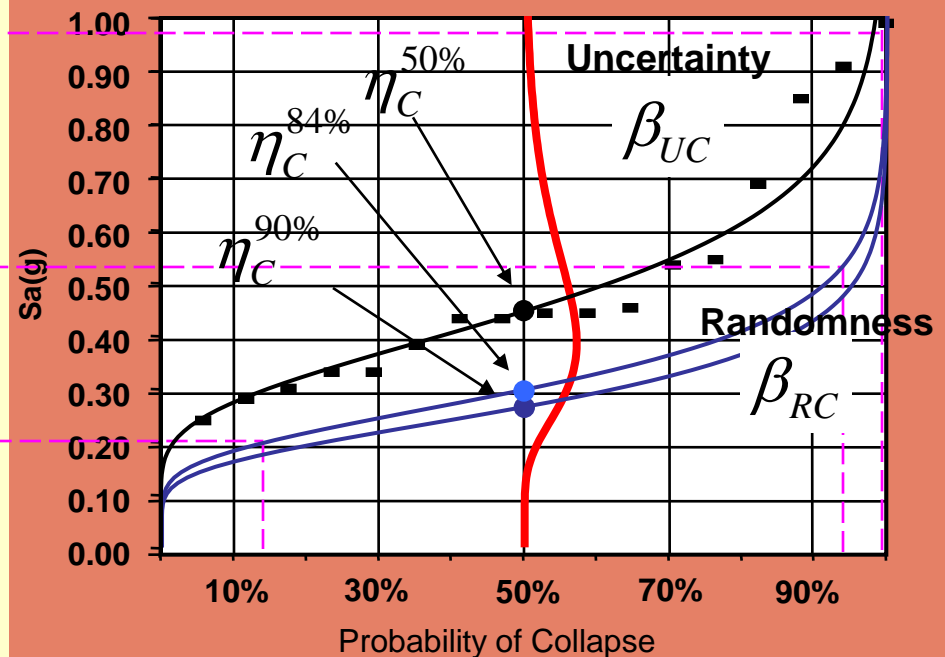
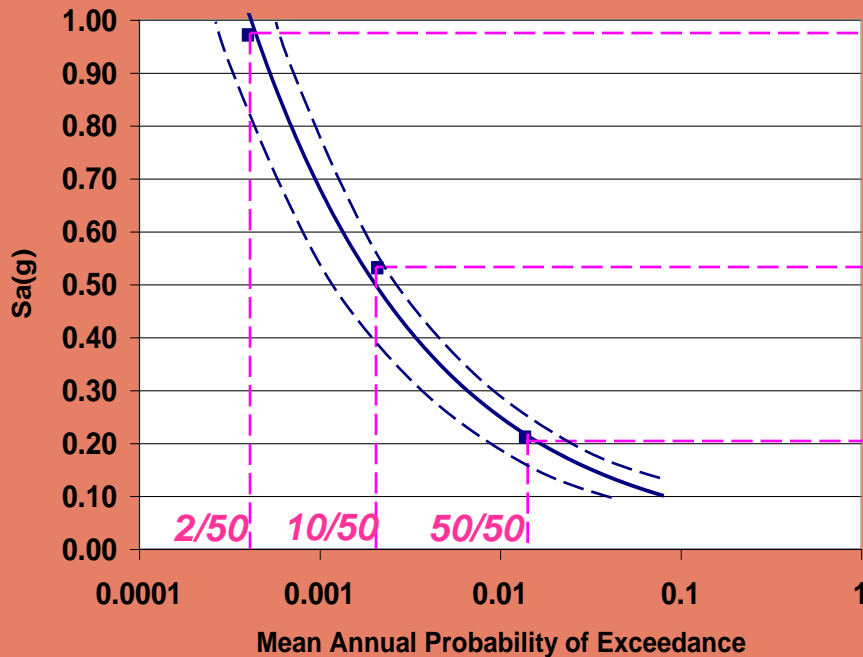
$$\bar{\lambda}(Sa)$$

Uncertainty

Collapse fragility curve with Y confidence

$$P_Y(C | Sa)$$

Mean Hazard Curve for VNHB, $T = 1.5$ sec.



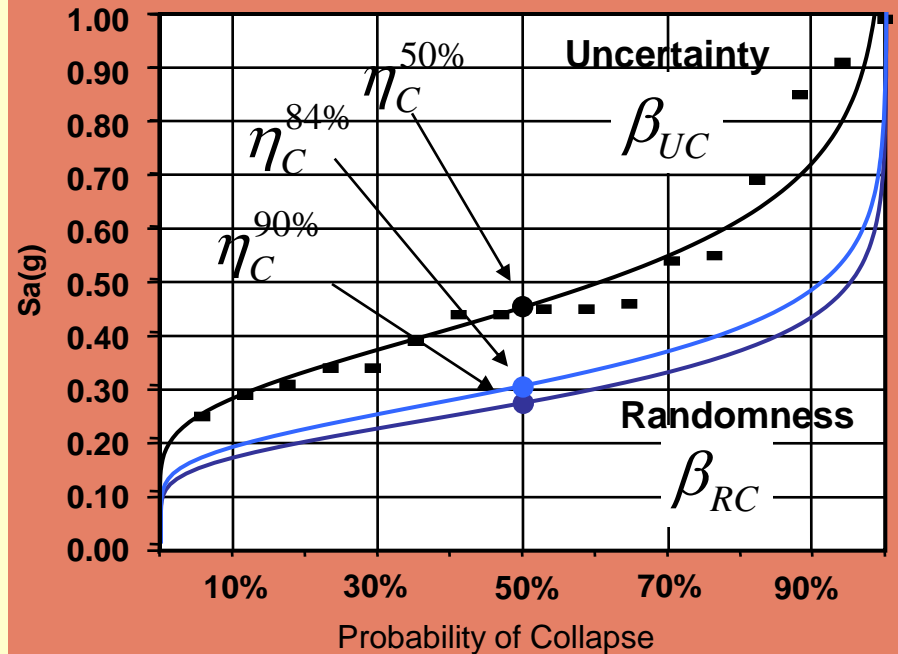
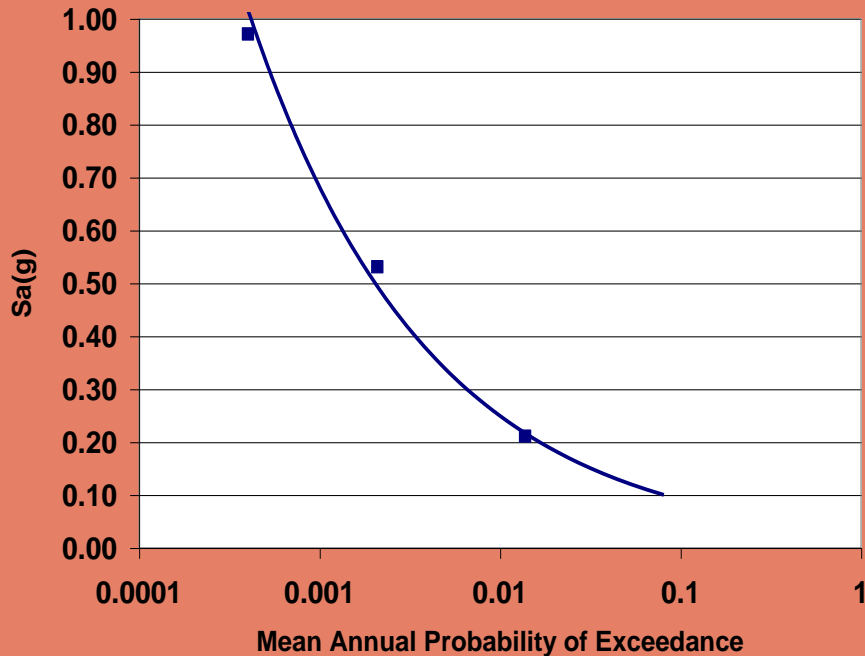
Assessment of MAF of collapse

Hazard Curve
 $\bar{\lambda}(Sa)$

$$\lambda_C^Y = \int_{Sa} P_Y(C | Sa) | d\bar{\lambda}_{Sa}(Sa) |$$

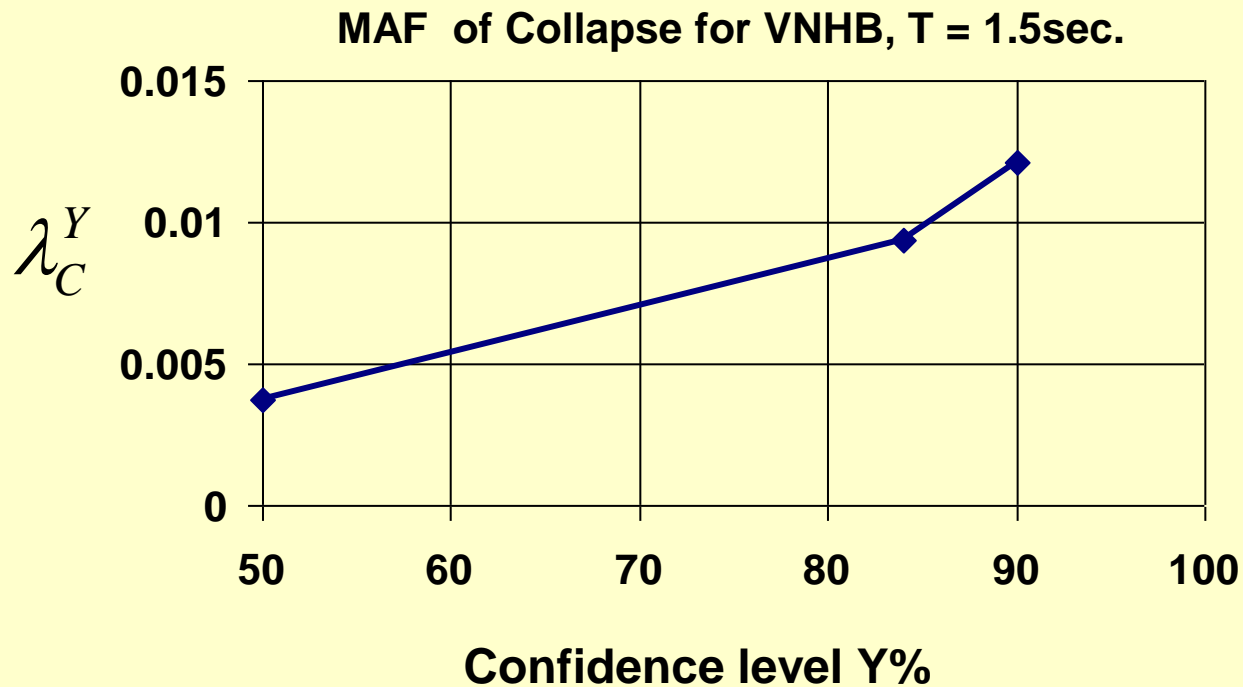
collapse fragility curve
 confidence
 $P_Y(C | Sa)$

Mean Hazard Curve for VNHB, T = 1.5 sec.

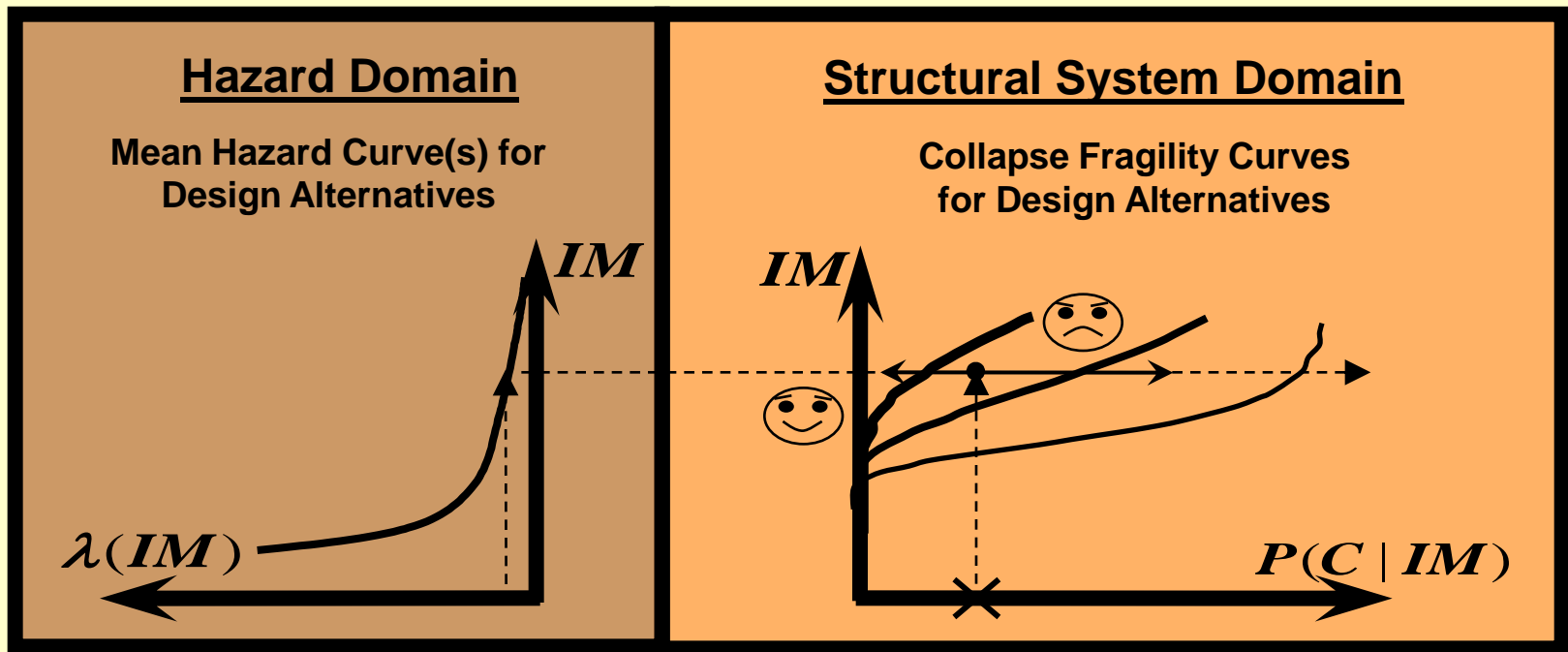


Assessment of MAF of collapse

$$\lambda_C^Y = \int_{Sa} P_Y(C | Sa) |d\bar{\lambda}_{Sa}(Sa)|$$



Performance-based design for collapse safety

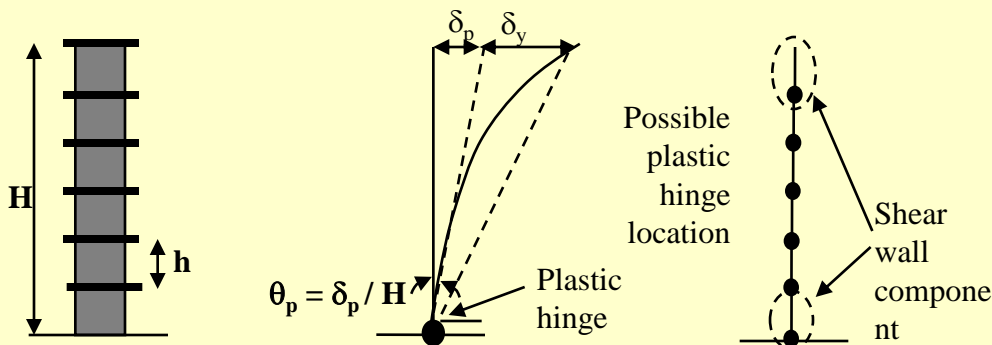
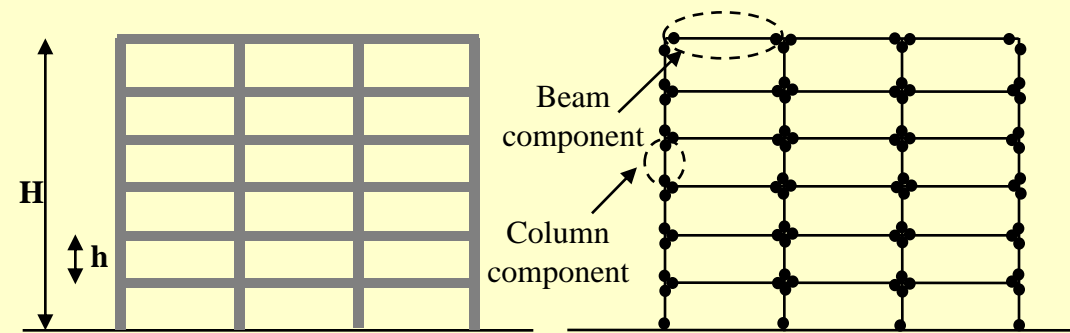


$$\lambda_C = \int_{S_a} P(C | s_a) |d\bar{\lambda}_{S_a}(S_a)|$$

Structural Parameters Matrix

Major Design Decision Parameters:

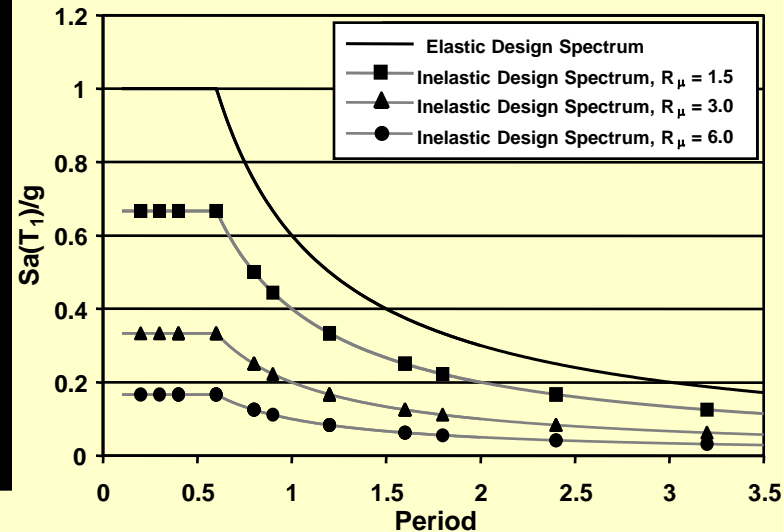
Generic Frame & Wall structures



Period and Yield Base Shear Coefficient

Design Response Spectra (MRF & SW)

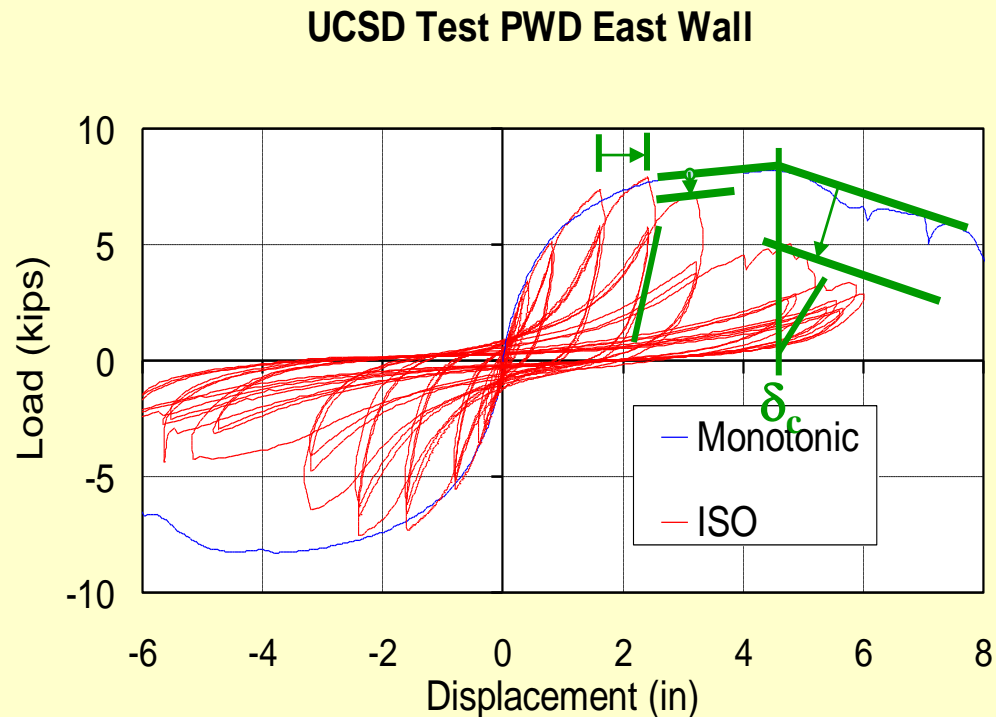
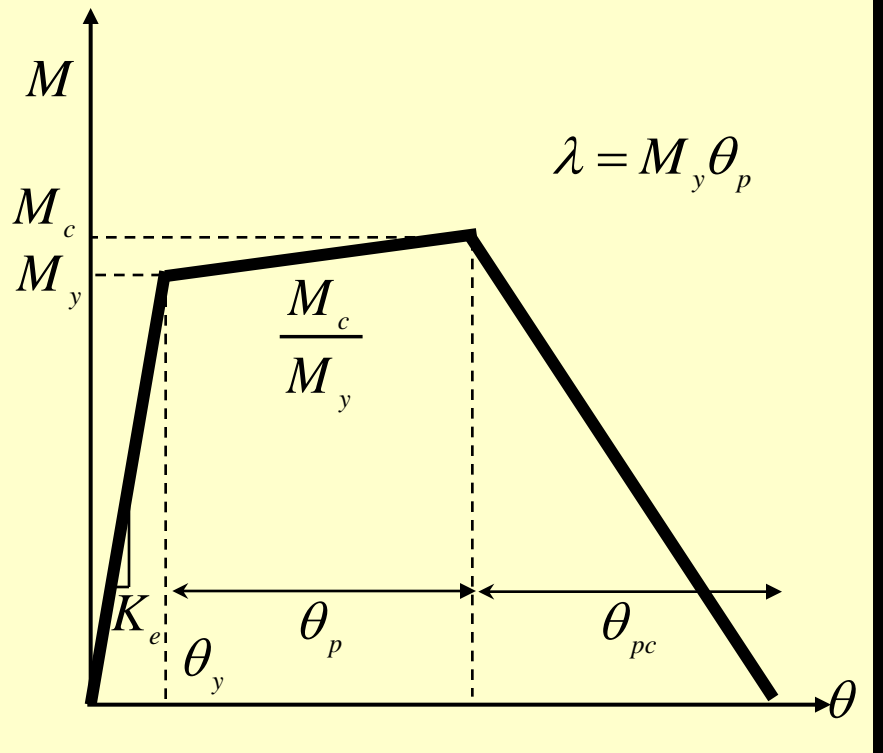
$\xi = 5\%$, Soil type D



Structural Parameters Matrix

Major Design Decision Parameters:

Component Model

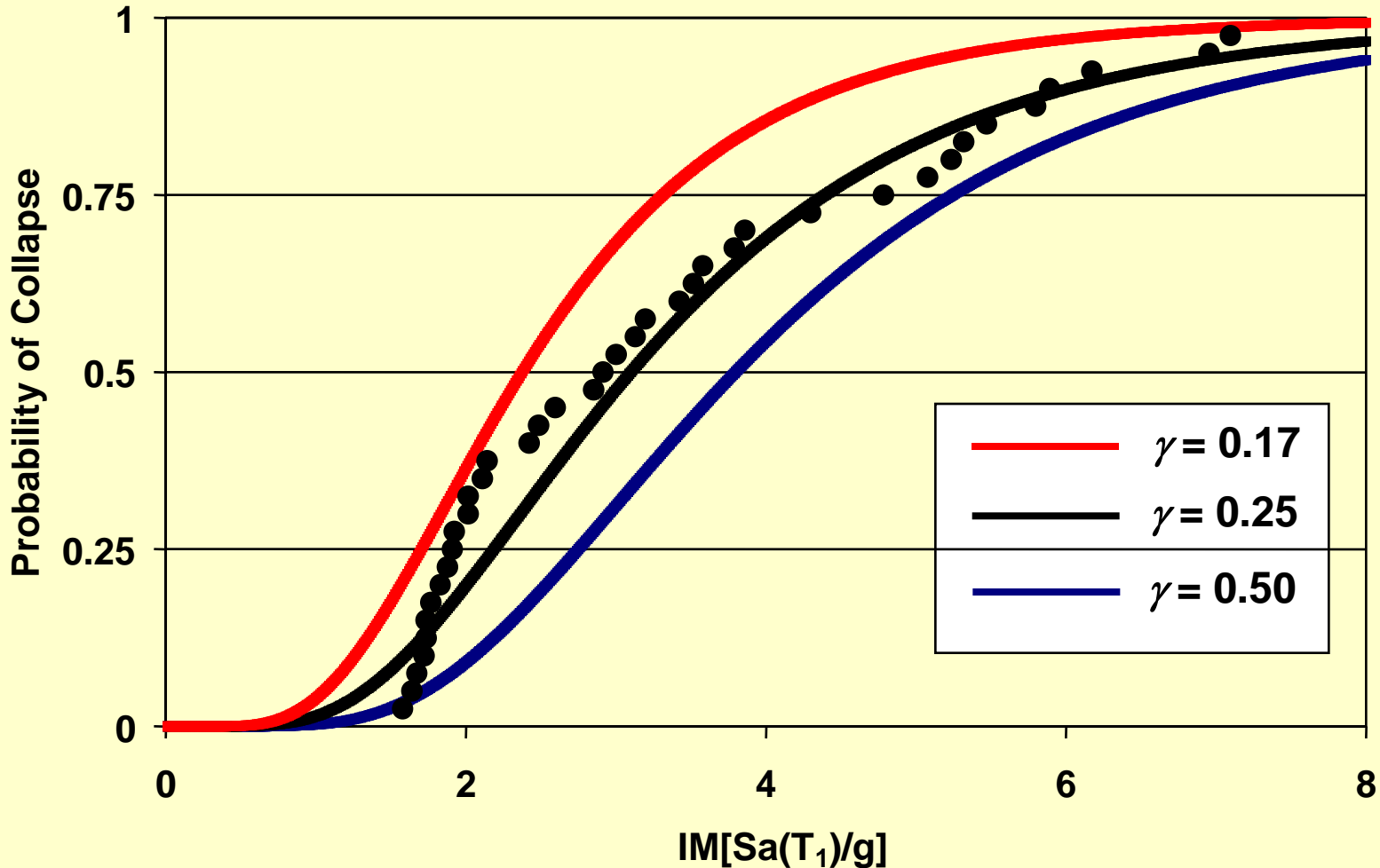


Development of collapse fragility curves

Collapse fragility curves (SW)

$N = 8, T = 0.8, \gamma = \text{var.}, \text{Stiff} = \text{Unif. Str.} = -0.05M_{y,\text{base}}, \xi = 0.05$

$\theta_p = 0.02, \theta_{pc}/\theta_p = 1, \lambda = 20, M_c/M_y = 1.1$

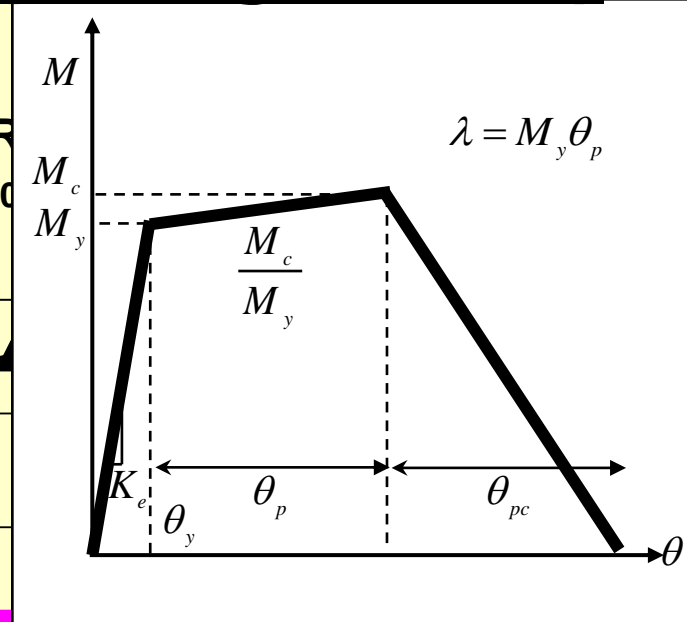
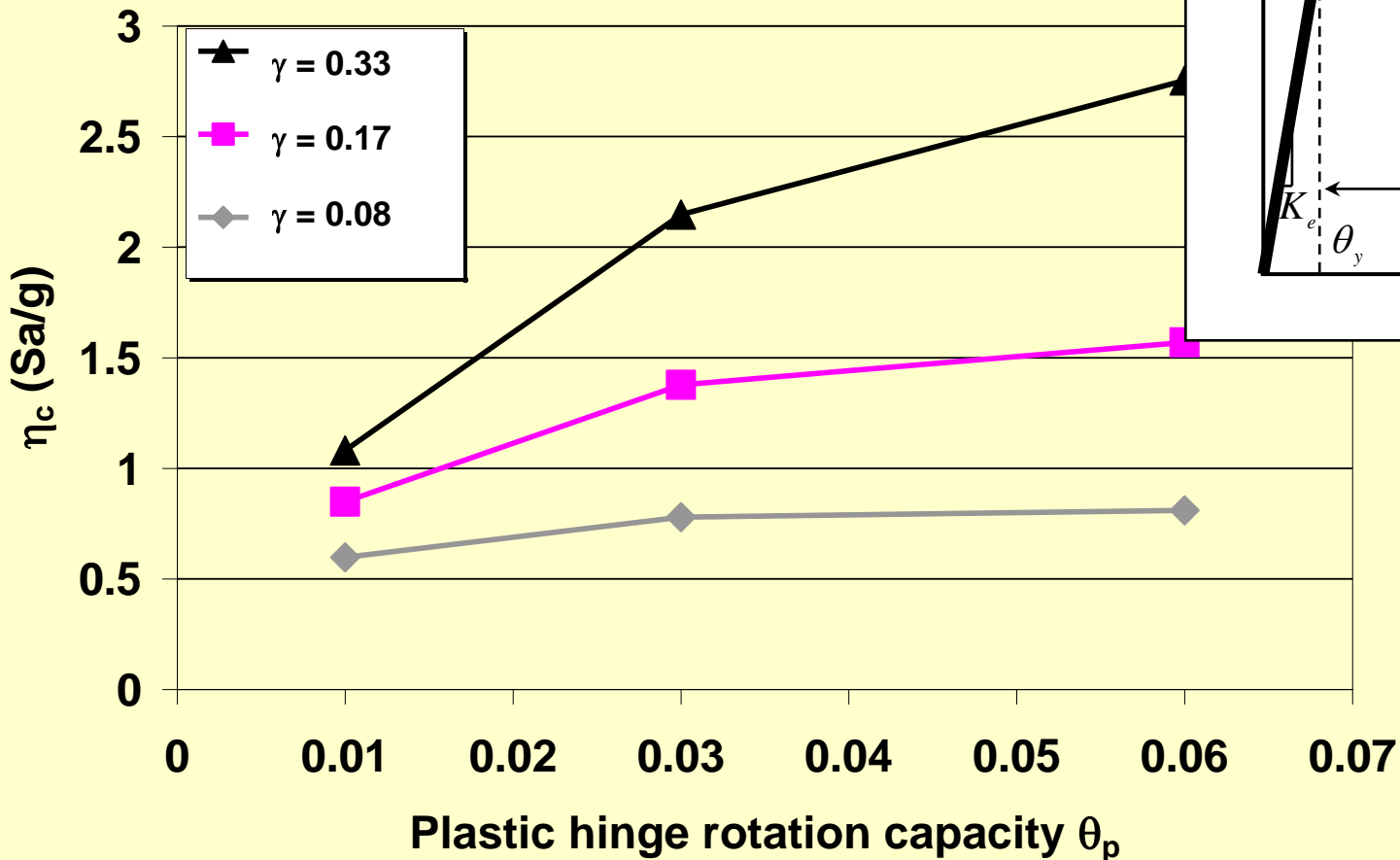


PBD for Collapse Safety (Design Aids)

Plastic Hinge Rotation Capacity Effect on η_c (MR)

$N = 8, T_1 = 1.2, \gamma = \text{var.}, \text{Stiff. \& Str.} = \text{Shear}, \text{SCB} = 2.4-1.2, \xi = 0.0$

$\theta_p = \text{var.}, \theta_{pc}/\theta_p = 5.0, \lambda = 20, M_c/M_y = 1.1$

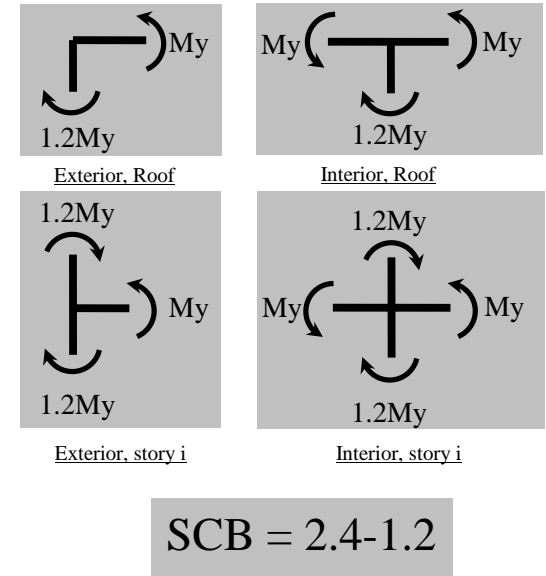
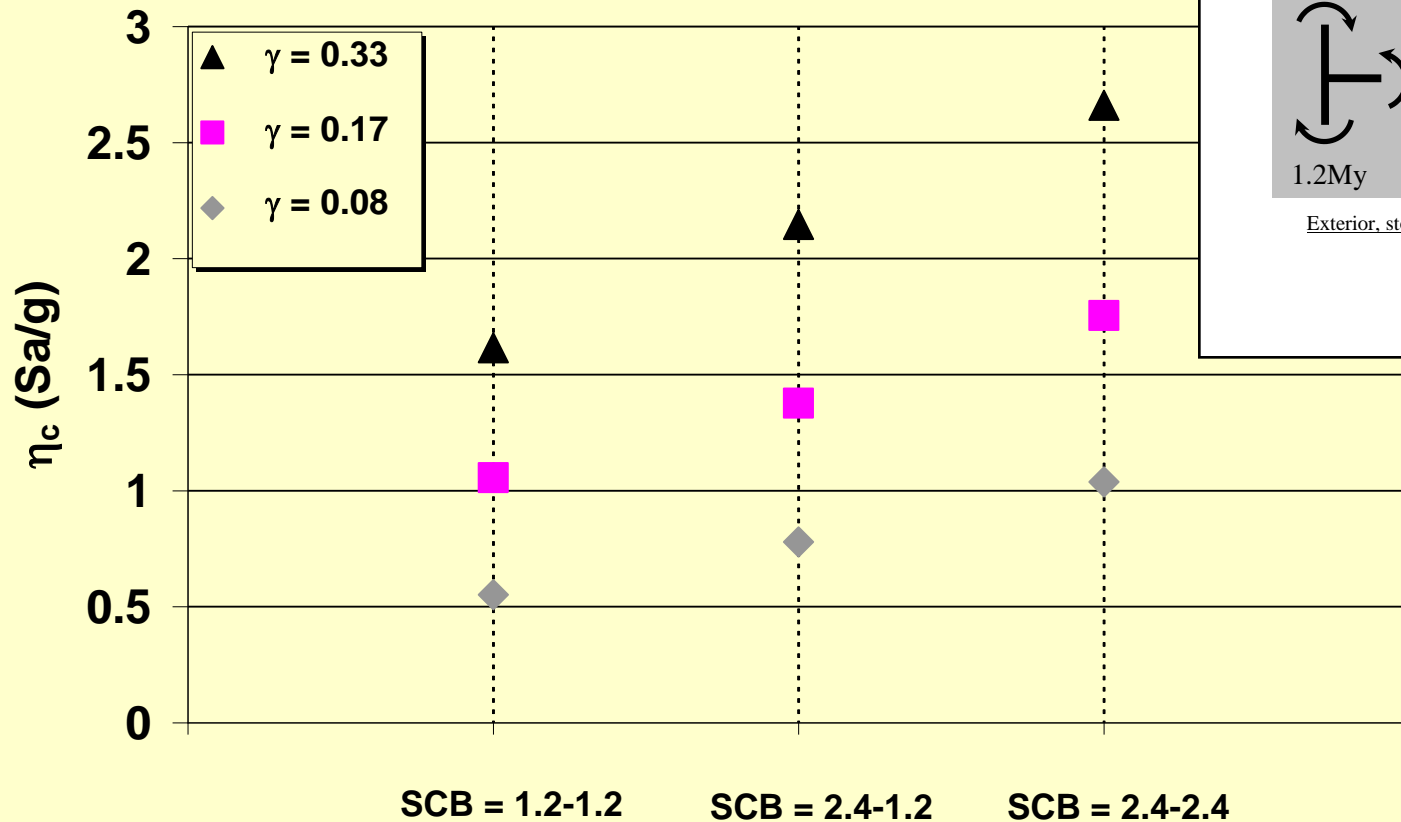


PBD for Collapse Safety (Design Aids)

Strong Column Beam Ratio Effect on η_c (MRF)

$N = 8, T_1 = 1.2, \gamma = \text{var.}, \text{Stiff. \& Str.} = \text{Shear}, \text{SCB} = \text{var.}, \xi = 0.05$

$\theta_p = 0.03, \theta_{pc}/\theta_p = 5.0, \lambda = 20, M_c/M_y = 1.1$

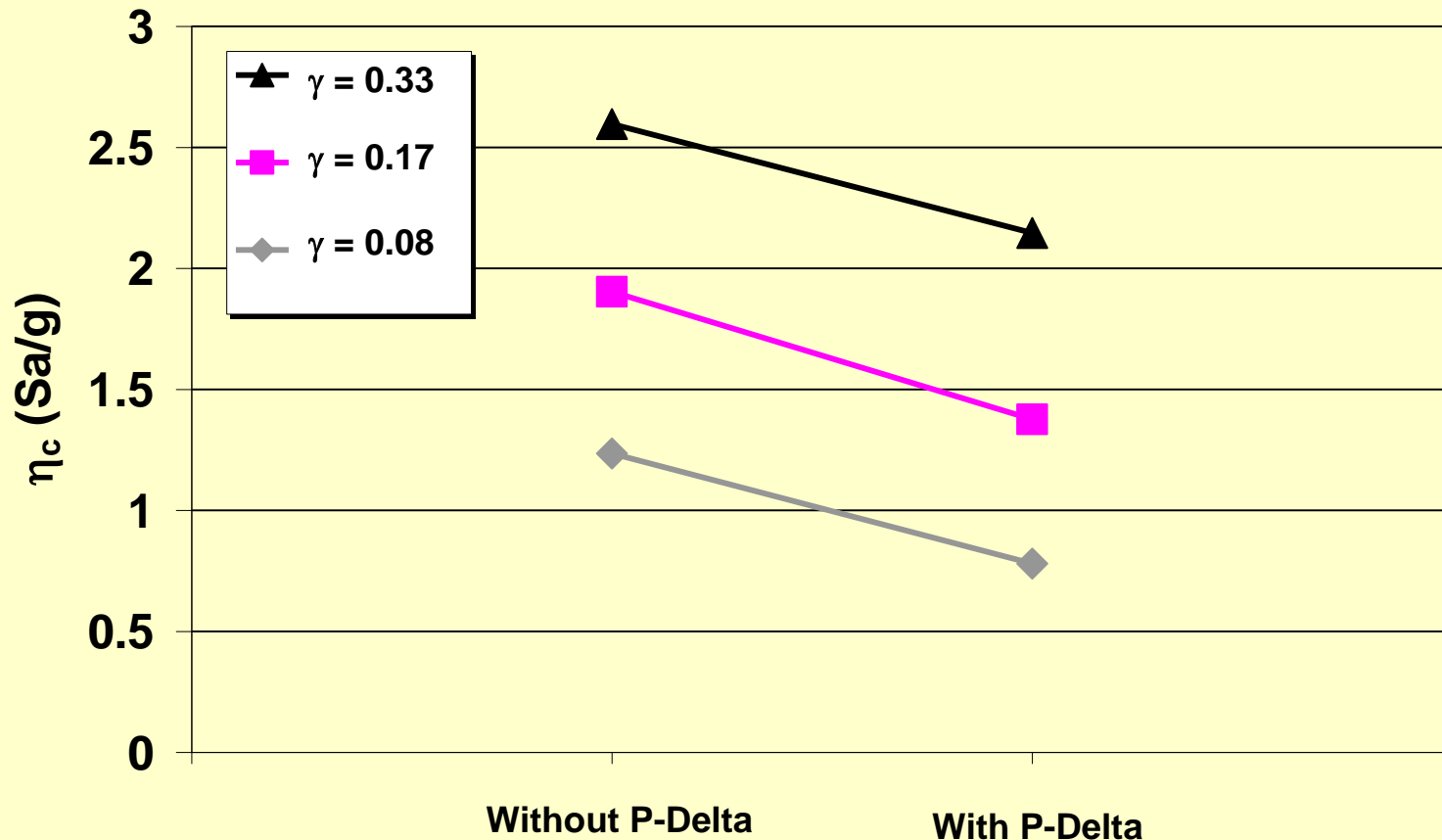


PBD for Collapse Safety (Design Aids)

P-Delta Effect on η_c (MRF)

N = 8, $T_1 = 1.2$, $\gamma = \text{var.}$, Stiff.&Str. = Shear, SCB = 2.4-1.2, $\xi = 0.05$

$\theta_p = 0.03$, $\theta_{pc}/\theta_p = 5.0$, $\lambda = 20$, $M_c/M_y = 1.1$



IM-EDP (Single story/floor)

Obtain relationship between IM and EDP for a specific story/floor level

IM-EDP (Maximum among all stories/floors)

Obtain relationship between IM and EDP for a maximum EDP among all stories/floors

EDP Profile

Obtain EDP at different story/floor levels given IM

Mean Loss Estimation (per floor/story)

Design Decision Support System (DDSS)

Mean Loss Estimation (Average among all stories/floors)

Design Decision Support System (DDSS)



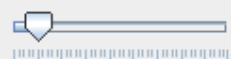


Probability of Collapse Estimation

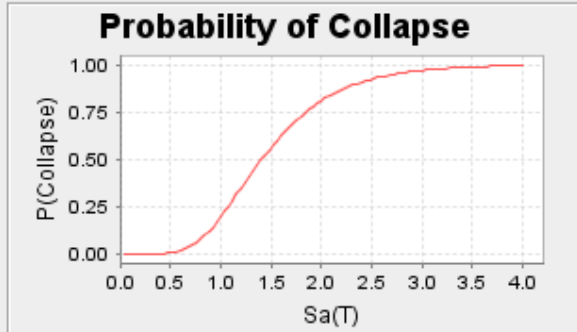
Obtain probability of collapse and its sensitivity to variation in structural parameters

[Comments and Suggestions](#)


<http://spee.eng.uci.edu>

Sensitivity of Probability of Collapse to Variation in Basic Structural Parameters


<p>Number of Stories</p>  <p>N = 9.0</p>	<p>First Mode Period</p>  <p>T = 1.35</p>	<p>Base Shear Coefficient</p>  <p>gamma = 0.222</p>	<p>Plastic Hinge Rotation Capacity</p>  <p>qp = 0.03</p>	<p>Post-Capping Rotation Capacity</p>  <p>qpc = 0.15</p>	<p>Cyclic Deterioration Parameter</p>  <p>lambda = 20</p>
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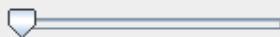
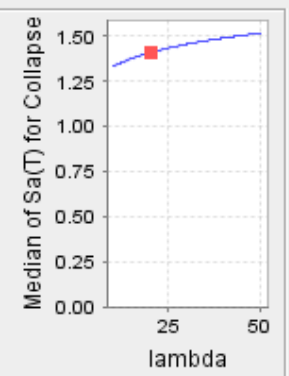
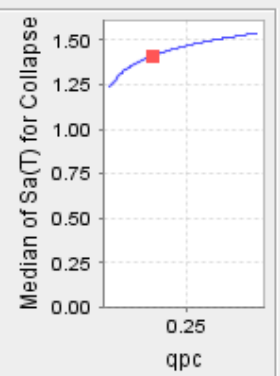
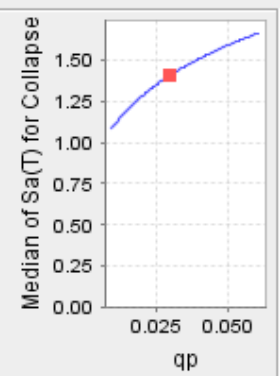
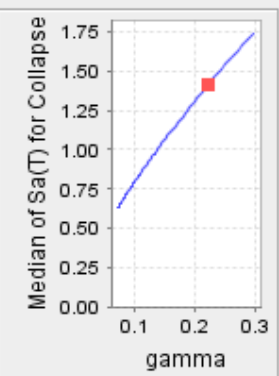
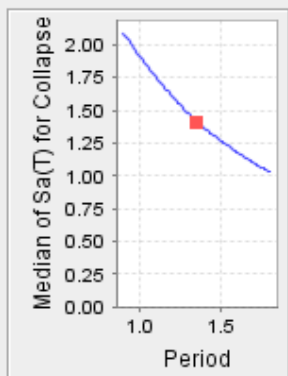
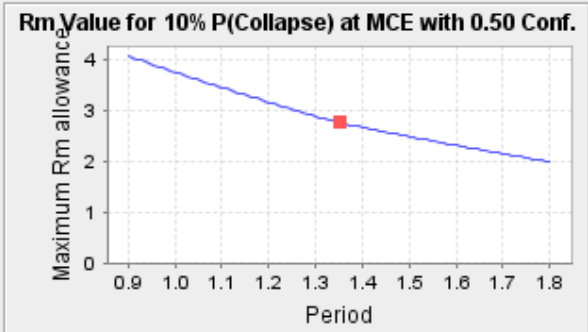
Disp. (Randomness) = 0.40



Disp. (Uncertainty) = 0.00



Confidence Level = 0.50

Concluding remarks

Can Collapse be Predicted?

- If we are capable of modeling deterioration characteristics of all important structural components
- If we are capable of modeling all collapse modes (local, story, global)
- If we are capable of predicting propagation of local collapse
- If we are capable of incorporating all “intangible” contributions that delay collapse

Concluding remarks

Given all constraints and assumptions

- Deterioration of structural components and P-Delta effects are fundamental causes of collapse.
- Including *Epistemic* uncertainty (i.e., confidence statements), increases the probability of collapse of a given building and increases the required strength and deformation capacity in conceptual design for collapse safety.
- Sensitivity of probability of collapse to variation of structural system and structural parameters were investigated
- A graphical tool to exercise Performance-Based Design for collapse safety is developed.

Acknowledgements

The research was supported by grants from NSF through Pacific Earthquake Engineering Research (PEER) Center, and Network for Earthquake Engineering Simulation Research (NEESR) Program, as well as the CUREE/Kajima Research Program. The support is well appreciated.

Thank You